

Exam CT4150 Plasticity Theory
Wednesday 11 June 2003, 9:00 – 12:00 hours

Problem 1

A frame consists of two columns and a roof beam which are rigidly connected (Figure 1). The frame is simply supported in point A and point B. The frame is loaded by three forces.

The columns and beam have the yield contour drawn in Figure 2. The following relation exists between the plastic moment M_p and the plastic normal force N_p .

$$N_p = \beta \frac{M_p}{a}$$

The influence of shear on the yield contour is neglected and buckling is not considered.

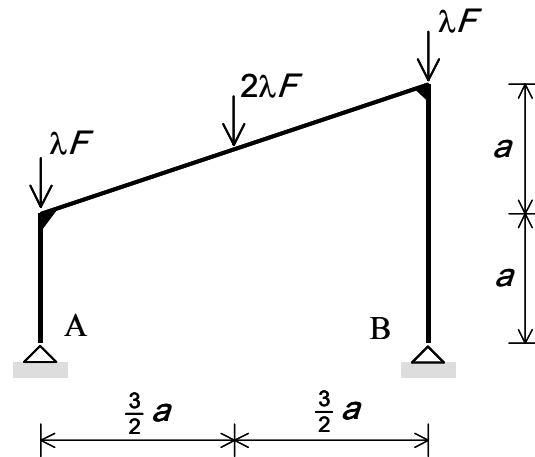


Figure 1. Simply supported frame

- a Assume $\beta \rightarrow \infty$. Determine the collapse load λF for every possible mechanism. Write the collapse loads as functions of M_p and a . Which mechanism is decisive? What is the corresponding collapse load? (1.5 points)
- b Assume $\beta \rightarrow \infty$. Draw the bending moment and normal force diagram for the whole structure at the moment of collapse (1 point).

- c Assume $\beta = 2$ for the columns and $\beta \rightarrow \infty$ for the roof beam. Choose one of the following problems (You need not do both) Use the decisive mechanism of problem 1a (2.5 points).

- Determine the largest lower-bound for λF .
- Determine the smallest upper-bound for λF .

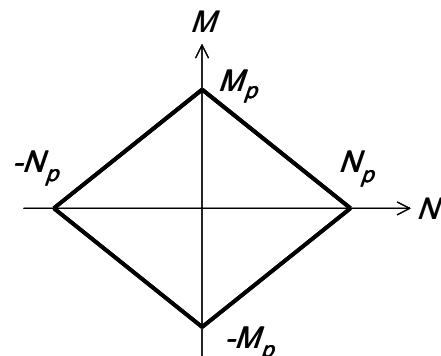


Figure 2. Yield contour

Problem 2

A square plate with an opening is simply supported at the edges (Figure 3). The plate carries an equally distributed load λf [kN/m²]. The plate is homogeneous and isotropic. The yield moment is m_p [kNm/m].

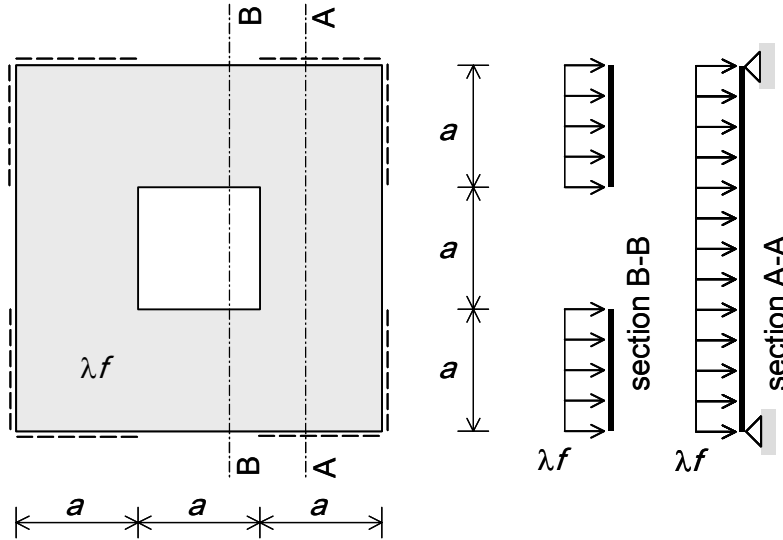


Figure 3. Simply supported square plate

- a We consider the yield line pattern of Figure 4. Determine an upper-bound for λf expressed in m_p and a (1 point).

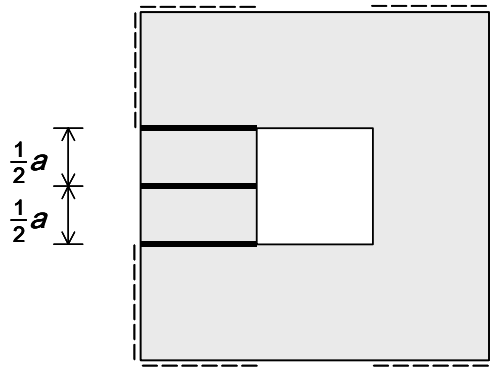


Figure 4. Yield line pattern of problem 2a

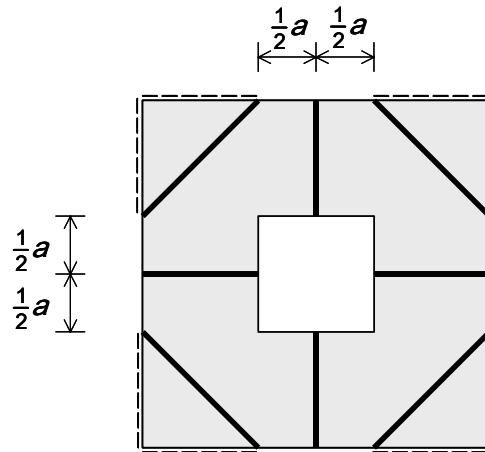
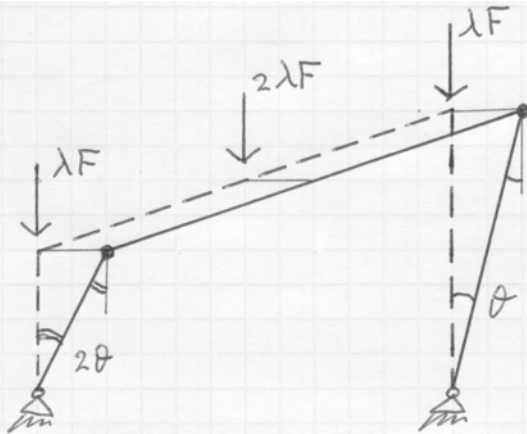


Figure 5. Yield line pattern of problem 2b

- b We consider the yield line pattern of Figure 5. Determine an upper bound for λf expressed in m_p and a (2 points).
- c Determine the largest lower-bound for λf using torsion free beams ($m_{xy} = 0$) in the x direction and y direction (2 points).

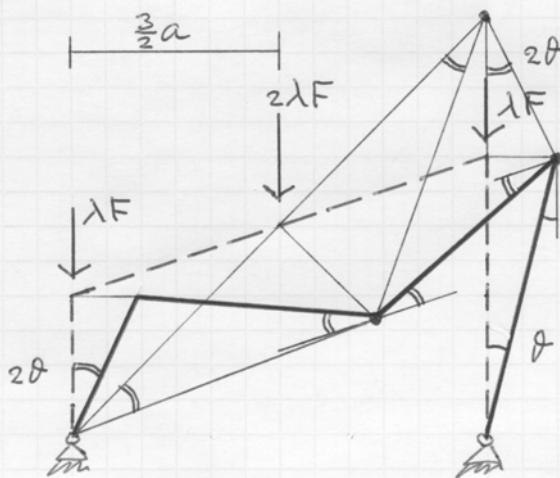
Answer to Problem 1a



$$A = 0$$

$$E = M_p(2\theta + \theta)$$

$$\lambda F = \text{indetermined}$$

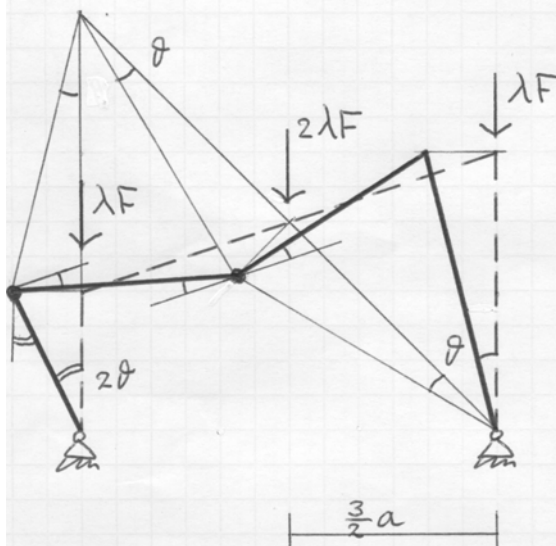


$$A = 2\theta \cdot \frac{3}{2}a \cdot 2\lambda F = 6\theta a \lambda F$$

$$E = M_p(4\theta + 3\theta) = 7\theta M_p$$

$$\lambda F = \frac{7}{6} \frac{M_p}{a} \approx 1,17 \frac{M_p}{a}$$

↑ decisive

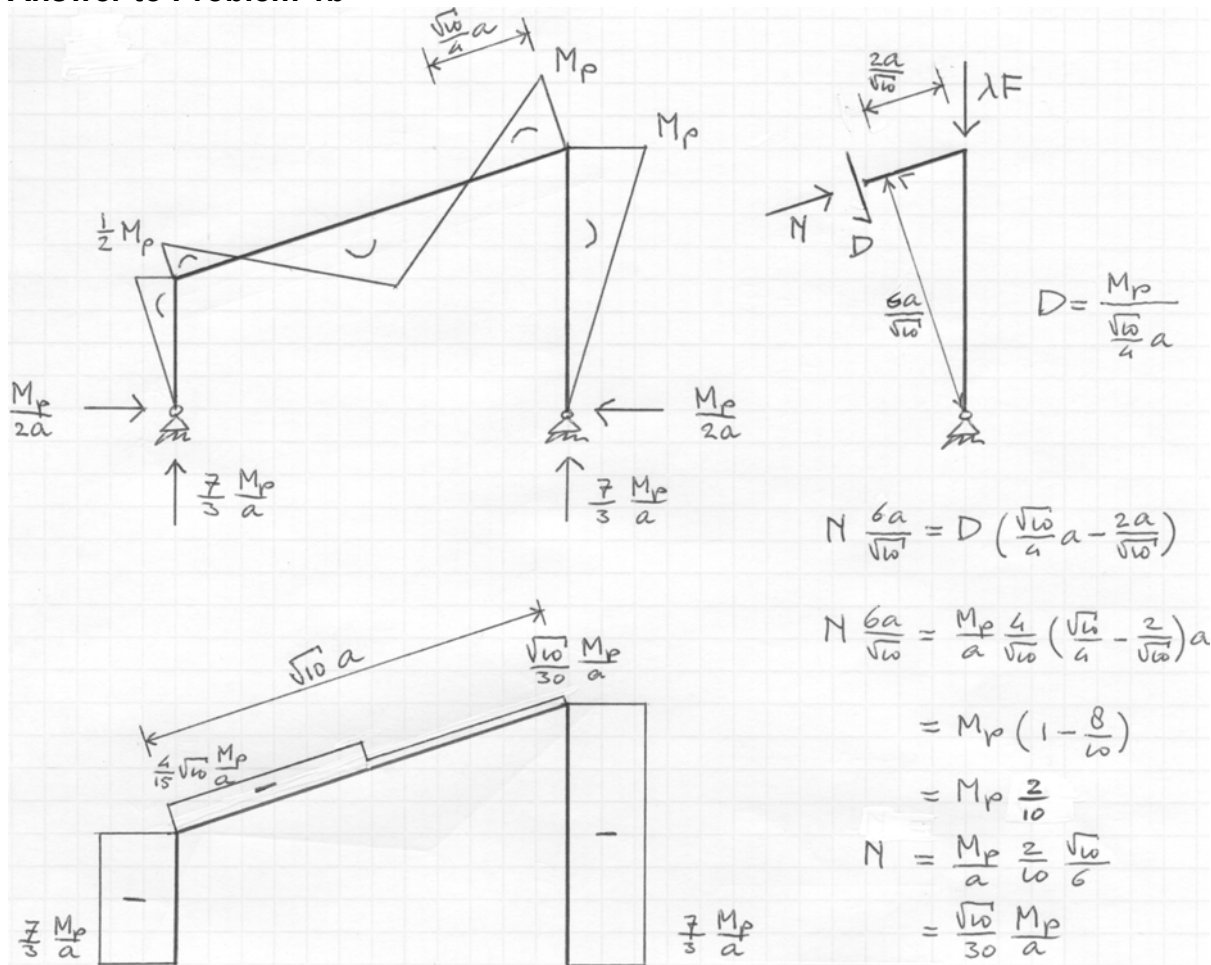


$$A = \theta \cdot \frac{3}{2}a \cdot 2\lambda F = 3\theta a \lambda F$$

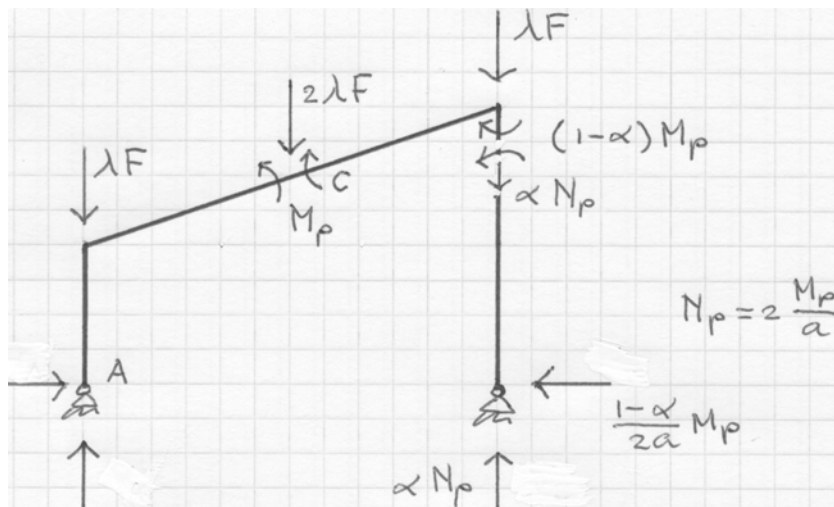
$$E = M_p(3\theta + 2\theta) = 5\theta M_p$$

$$\lambda F = \frac{5}{3} \frac{M_p}{a}$$

Answer to Problem 1b



Answer to problem 1c Lower-bound



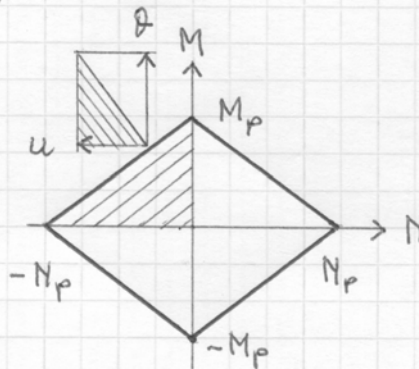
$$\begin{aligned}
 \sum \text{mom } A \\
 0 &= 2\lambda F \frac{3}{2}a + \lambda F 3a - \alpha N_p 3a \\
 &= 6a\lambda F - \alpha 2 \frac{M_p}{a} 3a \\
 &= 6a\lambda F - \alpha 6M_p \Rightarrow \lambda F = \alpha \frac{M_p}{a} \quad (1)
 \end{aligned}$$

$$\begin{aligned}
 \sum \text{mom } C \text{ right-hand side} \\
 M_p &= \alpha N_p \frac{3}{2}a - \frac{1-\alpha}{2a} M_p \frac{3}{2}a - \lambda F \frac{3}{2}a \\
 \frac{M_p}{\frac{3}{2}a} &= \alpha 2 \frac{M_p}{a} - \frac{M_p}{2a} + \alpha \frac{M_p}{2a} - \lambda F \\
 \left(\frac{2}{3} + \frac{1}{2}\right) \frac{M_p}{a} &= \frac{5}{2} \alpha \frac{M_p}{a} - \lambda F \Rightarrow \lambda F = -\frac{M_p}{a} \left(-\frac{7}{6} - \frac{5}{2}\alpha\right) \quad (2)
 \end{aligned}$$

$$\begin{aligned}
 (1) \text{ and } (2) \\
 \alpha = -\frac{7}{6} + \frac{5}{2}\alpha \Rightarrow -\frac{3}{2}\alpha = -\frac{7}{6} \Rightarrow \alpha = \frac{7}{9} \\
 \lambda F = \frac{7}{9} \frac{M_p}{a}
 \end{aligned}$$

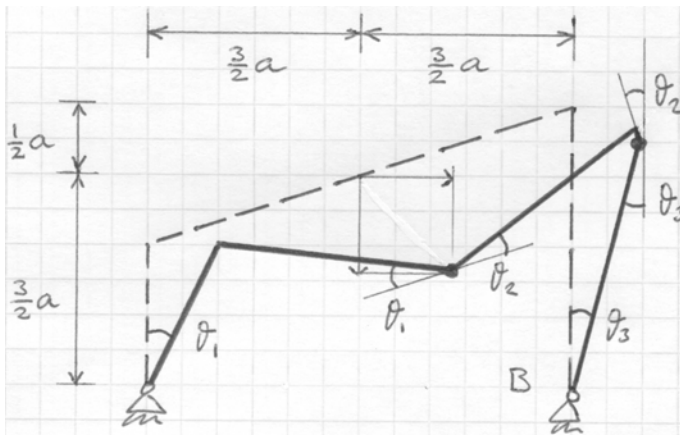
Answer to Problem 1c Upper-bound

normality



$$\frac{u}{\theta} = \frac{M_p}{N_p} = \frac{a}{\beta}$$

$$u = \theta \frac{a}{\beta} = \frac{1}{2} \theta a$$



horizontal displacement of point B

$$\theta_1 \frac{3}{2} a - \theta_2 \frac{1}{2} a - \theta_3 2a = 0$$

vertical displacement of point B

$$\theta_1 \frac{3}{2} a - \theta_2 \frac{3}{2} a - \frac{1}{2} a (\theta_2 + \theta_3) = 0$$

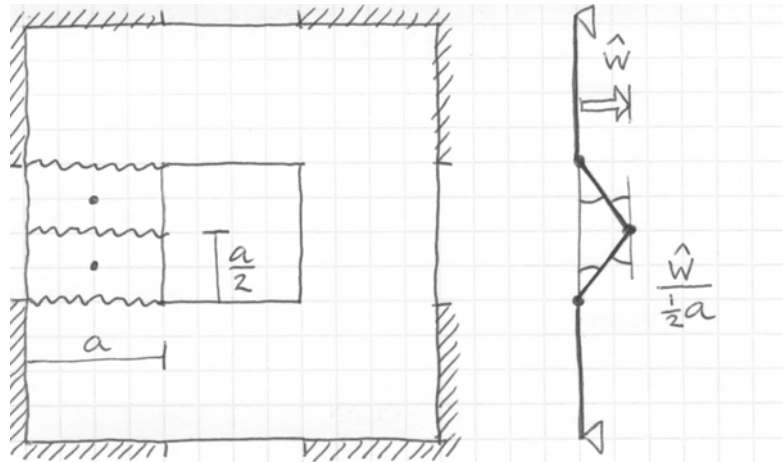
$$\begin{cases} \theta_1 = \frac{5}{3} \theta_3 \\ \theta_2 = \theta_3 \end{cases}$$

$$A = 2\lambda F \theta_1 \frac{3}{2} a + \lambda F \frac{1}{2} a (\theta_2 + \theta_3) = 6\lambda F \theta_3 a$$

$$E = M_p (\theta_1 + \theta_2) + M_p (\theta_2 + \theta_3) = \frac{14}{3} M_p \theta_3$$

$$A = E \Rightarrow \lambda F = \frac{7}{9} \frac{M_p}{a} \approx 0.78 \frac{M_p}{a}$$

Answer to Problem 2a



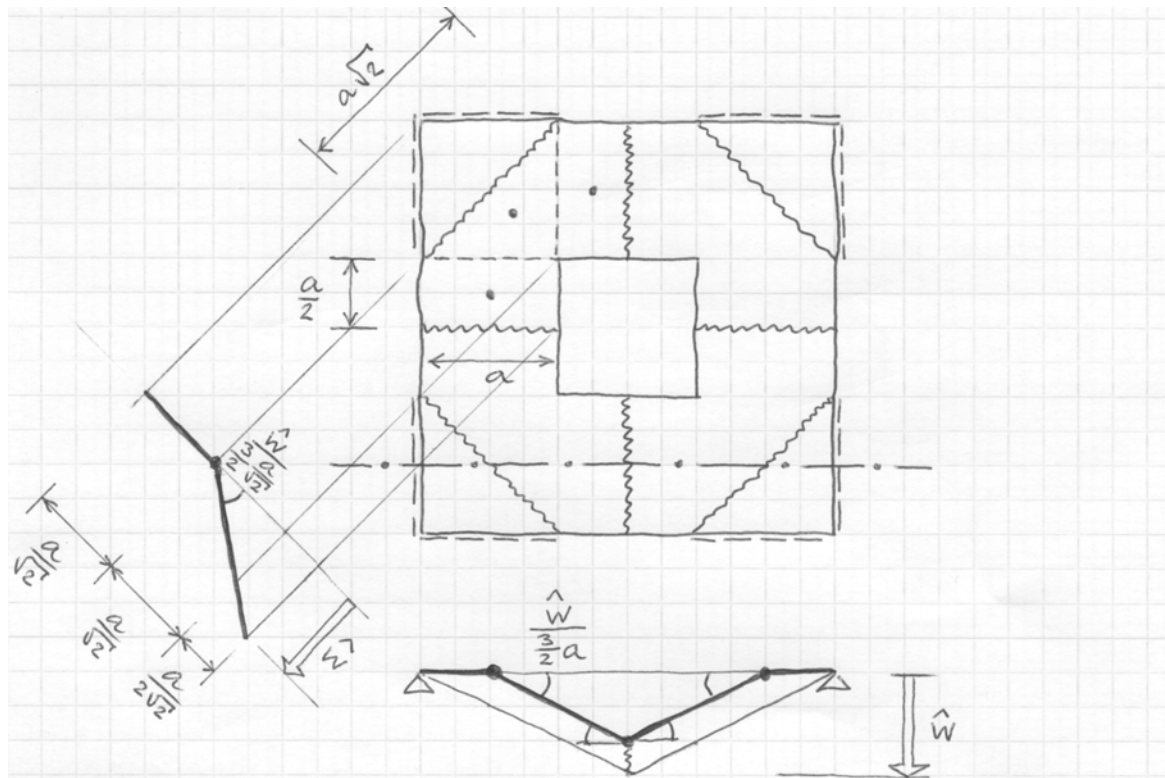
$$A = 2 \left(\frac{\hat{w}}{2} \lambda f a \frac{a}{2} \right) = \frac{1}{2} a^2 \hat{w} \lambda f$$

$$E = \frac{\hat{w}}{\frac{1}{2}a} m_p a + 2 \frac{\hat{w}}{\frac{1}{2}a} m_p a + \frac{\hat{w}}{\frac{1}{2}a} m_p a$$

$$= 8 \hat{w} m_p$$

$$A = E \Rightarrow \lambda f = 16 \frac{m_p}{a^2}$$

Answer to Problem 2b



$$A = 4 \left[\frac{\hat{w}}{2} \lambda f a \frac{a}{2} + \frac{\hat{w}}{2} \lambda f a \frac{a}{2} + \frac{2}{9} \hat{w} \lambda f \frac{a a}{2} \right] = \frac{22}{9} a^2 \hat{w} \lambda f$$

$$E = 4 \left[\frac{\hat{w}}{\frac{3}{2} a} a \sqrt{2} m_p \right] + 4 \left[2 \frac{\hat{w}}{\frac{3}{2} a} a m_p \right] = \frac{32}{3} \hat{w} m_p$$

$$\lambda f = \frac{48}{11} \frac{m_p}{a^2} \approx 4,36 \frac{m_p}{a^2}$$

Answer to Problem 2c

