

**Exam CT4150 Plastic Analysis of Structures**

Tuesday 21 August 2007, 14:00 – 17:00 hours

**Problem 1**

A frame consists of two columns, one beam and two brackets for a travelling crane (Fig. 1). The yield strengths of all members and joints are  $M_p$ . The columns are fixed in the foundation. The frame is loaded by a distributed load  $q$  and two vertical forces  $F = 10qa$ . The following relation exists between the plastic moment  $M_p$  and the plastic normal force  $N_p$  (Fig. 2).

$$N_p = \beta \frac{M_p}{a}$$

The influence of shear on the yield contour is neglected. Buckling and second order effects are not considered.

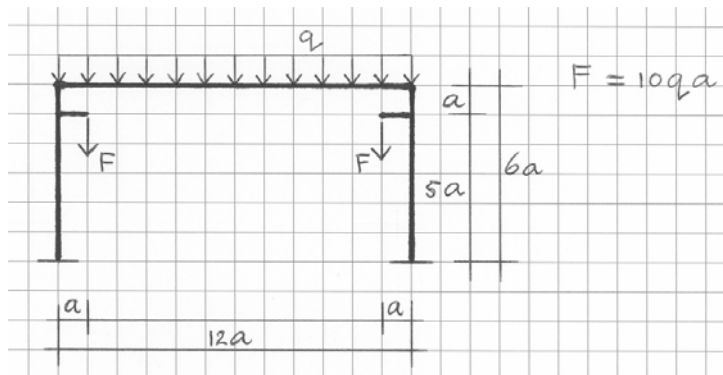


Figure 1. Frame

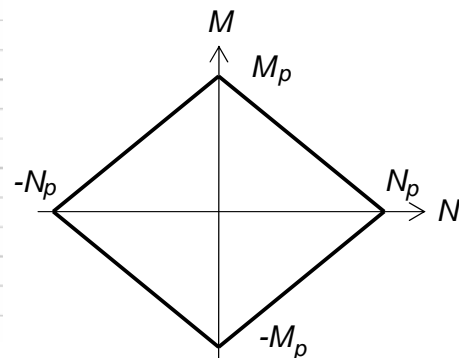


Figure 2. Yield contour

- a Assume  $\beta \rightarrow \infty$ . Determine the collapse load  $q$  for all important mechanisms. Write the collapse loads as functions of  $M_p$  and  $a$ . What is the decisive collapse load? (1.5 points)
- b Assume  $\beta \rightarrow \infty$ . Draw the bending moment diagram and normal force diagram for the structure at the moment of collapse. (1 point)
- c Assume  $\beta = 64$ . Choose one of the following problems (You need not do both).
  - Determine the largest lower-bound for  $F$ .
  - Determine the smallest upper-bound for  $F$ .

If you choose the upper-bound you only need to write down the equations and not solve the equations. (2 points)

## Problem 2

A plate has hinged edges and free edges (Fig. 3). The plate carries a perpendicular evenly distributed load  $q$  [kN/m<sup>2</sup>]. The plate is homogeneous. There is only reinforcement near the bottom face of the plate in the  $x$  and  $y$  directions.  $m_{px} = m_{py} = m_p$  and  $m'_{px} = m'_{py} = 0$ .

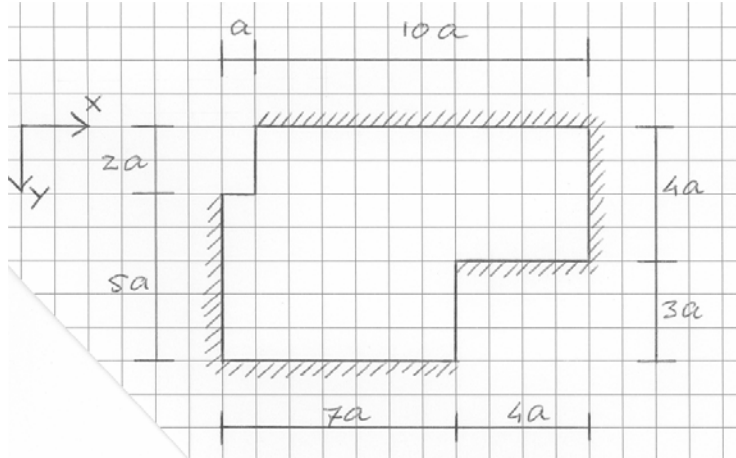


Figure 3. Plate with dimensions

- a Consider the yield line patterns of Figure 4. Which of these patterns give kinematically possible mechanisms. (1 point)

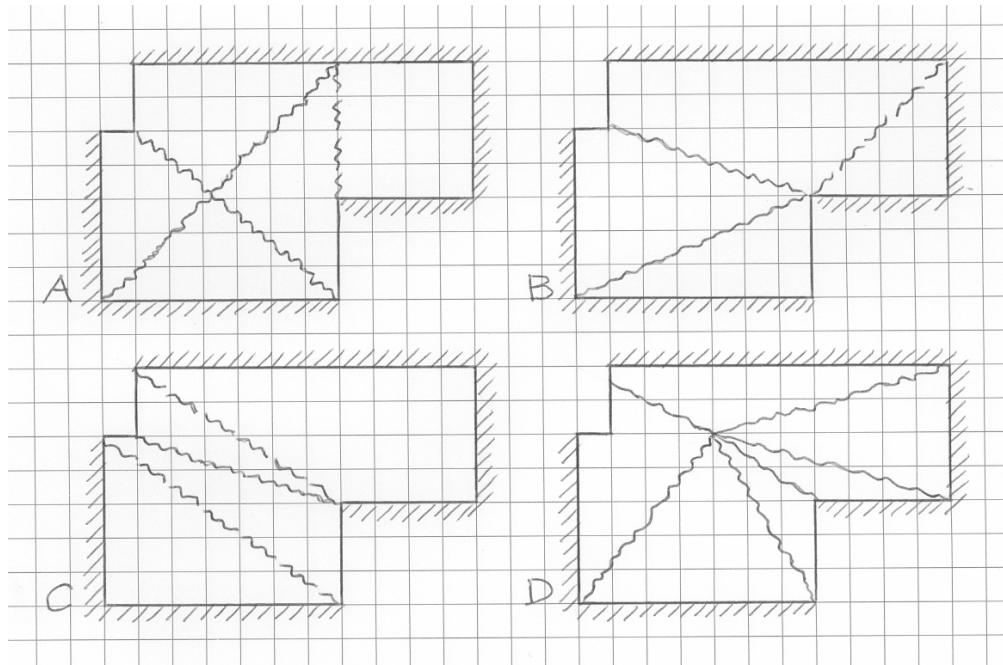


Figure 4. Proposed yield line patterns

- b Consider the yield line pattern of Figure 5. Determine an upper bound for  $q$  expressed in  $m_p$  and  $a$ . (1.5 points)

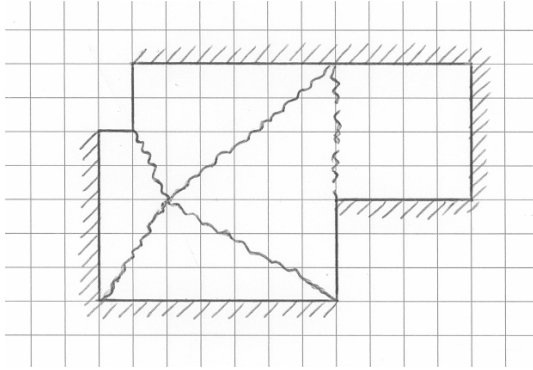


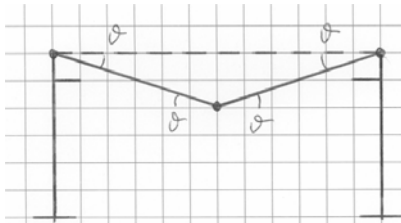
Figure 5. Actual yield line pattern

- c Determine the largest lower-bound for  $q$  using torsion free beams ( $m_{xy} = 0$ ) in the  $x$  direction and  $y$  direction. You only need to write down all equations and not solve the equations. (1.5 points)

### Problem 3

- a When we do not choose the right mechanism in an upper-bound analysis we will notice that when plotting the moments. In what way? Choose A, B, C or D. (0.5 point)
- A – The moment distribution will not be in equilibrium.
  - B – Somewhere a moment will be larger than the moment capacity.
  - C – There will not be sufficient plastic hinges to plot all of the moment distribution.
  - D – The largest moment does not occur where the shear force is zero.
- b A ductile structure has a convex yield contour. This is important because ... Choose A, B, C or D. (0.5 point)
- A – it proves that the solution of plastic problems is independent of the load path.
  - B – structures need to be able to resist unexpected accidents and terrorist attacks.
  - C – it determines the proportions of the plastic deformation in finite element analyses.
  - D – therefore we need to check just a few load combinations.
- c In engineering practice, elastic analysis is used much more than plastic analysis (upper-bound, lower-bound) What is the reason for this? Choose A, B, C or D. (0.5 point)
- A – There is little software available for plastic analysis.
  - B – Codes of practice do not allow plastic deformations.
  - C – Plastic analysis gives information on serviceability limit states only.
  - D – Elastic analysis is faster than plastic analysis.

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Answer to Problem 1a

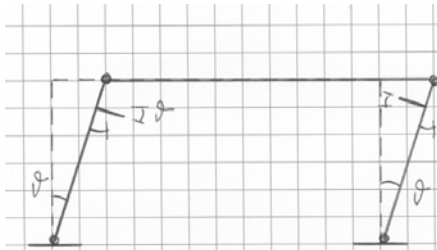


$$E = M_p \theta + M_p (\theta + \theta) + M_p \theta$$

$$= 4 M_p \theta$$

$$A = q \cdot 6a \cdot 3a = 18 q a^2 \theta$$

$$E = A \Rightarrow q = \frac{2}{9} \frac{M_p}{a^2}$$

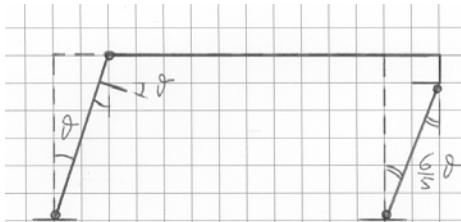


$$E = M_p \theta + M_p \theta + M_p \theta + M_p \theta$$

$$= 4 M_p \theta$$

$$A = F \theta a - F \theta a = 0 q$$

$$E = A \Rightarrow q = \infty$$



$$E = M_p \theta + M_p \theta + M_p \frac{6}{5} \theta + M_p \frac{6}{5} \theta$$

$$= \frac{22}{5} M_p \theta$$

$$A = F \theta a = 10 q a^2 \theta$$

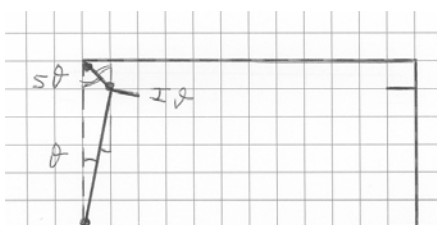
$$E = A \Rightarrow q = \frac{22}{50} \frac{M_p}{a^2}$$



$$E = M_p \theta$$

$$A = F \theta a = 10 q a^2 \theta$$

$$E = A \Rightarrow q = \frac{1}{10} \frac{M_p}{a^2}$$

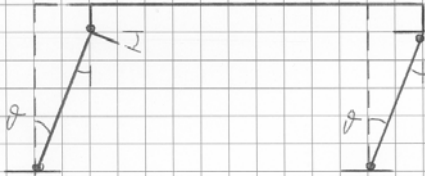


$$E = M_p \theta + M_p 5 \theta + M_p (\theta + 5 \theta)$$

$$= 12 M_p \theta$$

$$A = F \theta a = 10 q a^2 \theta$$

$$E = A \Rightarrow q = \frac{13}{10} \frac{M_p}{a^2}$$

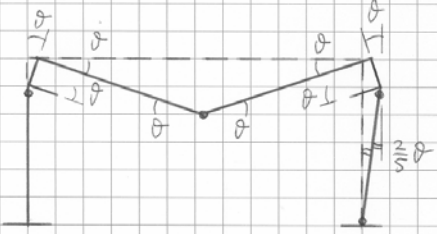


$$E = M_p \theta + M_p \theta + M_p \theta + M_p \theta$$

$$= 4 M_p \theta$$

$$A = F \theta a = 10 q a^2 \theta$$

$$E = A \Rightarrow q = \frac{4}{10} \frac{M_p}{a^2}$$



$$E = M_p \theta + M_p (\theta + \theta) + M_p (\theta + \frac{2}{5} \theta)$$

$$+ M_p \frac{2}{5} \theta = \frac{24}{5} M_p \theta$$

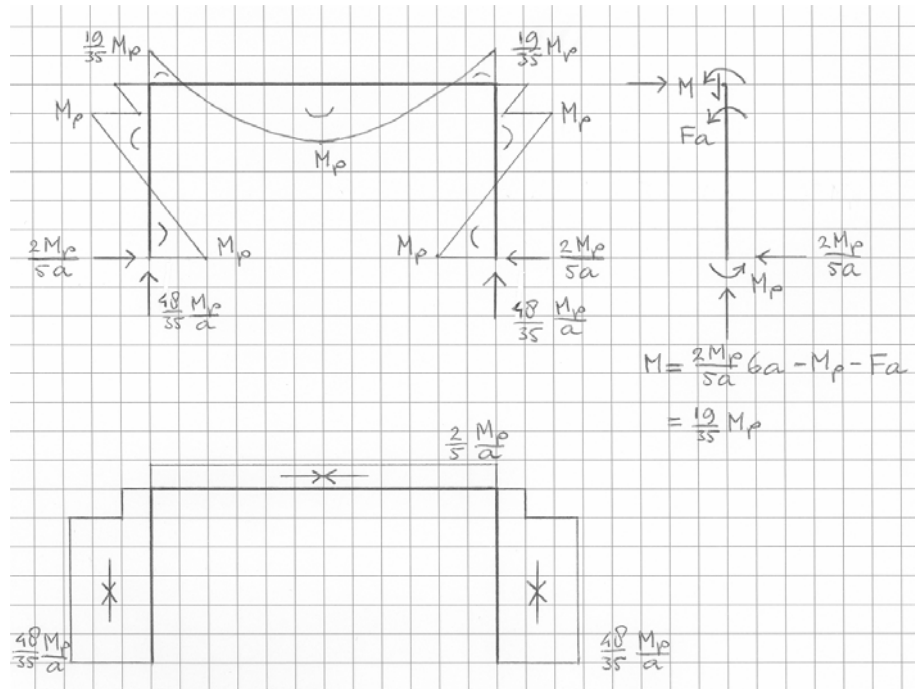
$$A = 2 F \theta a + 2 q 6a \theta 3a$$

$$= 56 q a^2$$

$$E = A \Rightarrow q = \frac{3}{35} \frac{M_p}{a^2} \text{ decisive}$$

$$0,0857$$

### Answer to Problem 1b



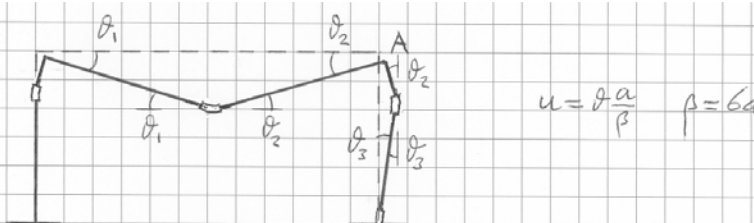
### Answer to problem 1c Lower-bound

$$M = \alpha \frac{48}{35} \frac{M_p}{a} = (1-\alpha) M_p = (1-\alpha) \beta \frac{M_p}{a}$$

$$\alpha \frac{48}{35} = (1-\alpha) \beta \Rightarrow \alpha = \frac{\beta}{\frac{48}{35} + \beta} = \frac{140}{143}$$

$$q_1 = \alpha \frac{3}{35} \frac{M_p}{a^2} = \frac{140}{143} \frac{3}{35} \frac{M_p}{a^2} = \frac{12}{143} \frac{M_p}{a^2} = 0,0839 \frac{M_p}{a^2}$$

### Answer to Problem 1c Upper-bound



displacement of point A  $\rightarrow$

$$\theta_1 a - (\theta_1 + \theta_2) \frac{a}{\beta} = \theta_3 5a - \theta_2 a$$

$$F = 10 q_1 a$$

displacement of point A  $\downarrow$

$$\theta_1 \frac{a}{\beta} + \theta_1 6a - \theta_2 6a = (\theta_2 + \theta_3) \frac{a}{\beta} + \theta_3 \frac{a}{\beta}$$

$$E = M_p \theta_1 + M_p (\theta_1 + \theta_2) + M_p (\theta_2 + \theta_3) + M_p \theta_3$$

$$A = F(\theta_1 \frac{a}{\beta} + \theta_1 a) + q_1 6a (\theta_1 \frac{a}{\beta} + \theta_1 3a) +$$

$$+ q_1 6a (\theta_1 \frac{a}{\beta} + \theta_1 6a - \theta_2 3a) + F(\theta_3 \frac{a}{\beta} + (\theta_2 + \theta_3) \frac{a}{\beta} + \theta_2 a)$$

$$E = A$$

the solution of the equations is

$$\theta_1 = \frac{8809}{8791} \theta_2$$

$$q_1 = \frac{1532}{18143} \frac{M_p}{a^2} = 0,0844 \frac{M_p}{a^2}$$

$$\theta_3 = \frac{3465}{8791} \theta_2$$

### Answer to Problem 2a

Kinematically possible is pattern D only.

### Answer to Problem 2b

$$E = m_p \frac{w}{2a} 3a + m_p \frac{w}{3a} 2a + m_p \frac{w}{5a} 3a + m_p \frac{w}{3a} 5a +$$

$$+ m_p \frac{w}{5a} 4a + m_p \frac{w}{4a} 5a + m_p \frac{w}{2a} 2a + m_p \frac{w}{4a} a$$

$$= m_p \frac{116}{15} w$$

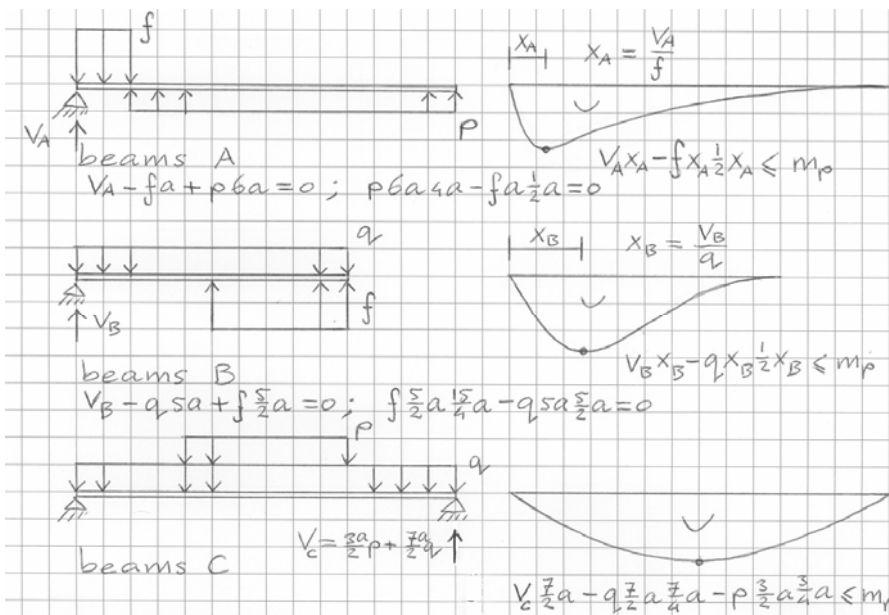
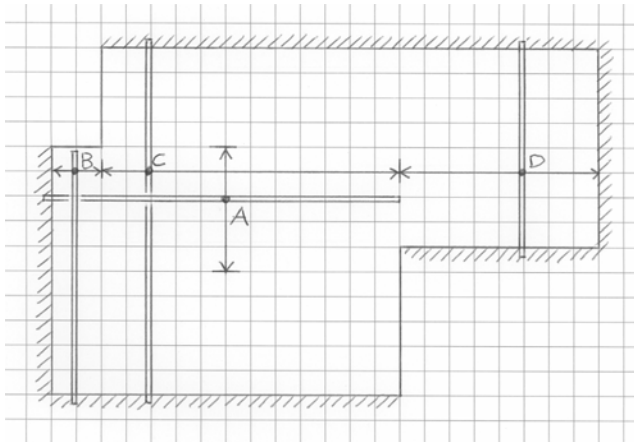
$$A = q \frac{1}{2} 7a 3a \frac{1}{3} w + q \frac{1}{2} 7a 5a \frac{1}{3} w + q \frac{1}{2} 7a 4a \frac{1}{3} w -$$

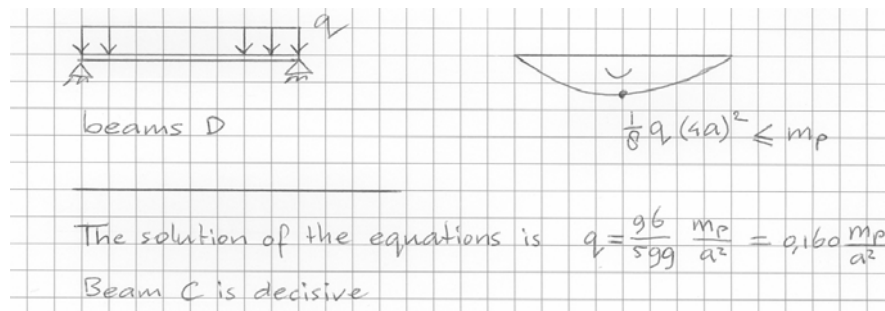
$$- q \frac{1}{2} 1a 2a \frac{1}{6} w + q \frac{1}{2} 7a 2a \frac{1}{3} w - q \frac{1}{2} 2a a \frac{1}{6} w$$

$$= q a^2 16 w$$

$$E = A \Rightarrow q = \frac{29}{60} \frac{m_p}{a^2}$$

### Answer to Problem 2c





### Answer to Problem 3a

B is correct.

### Answer to Problem 3b

D is correct.

### Answer to Problem 3c

A is correct.