Delft University of Technology

Faculty of Civil Engineering and Geosciences Structural Mechanics Section

Exam CIE4150 Plastic Analysis of Structures Thursday 16 June 2022, 13:30 – 16:30 hours

Write your <u>name</u> and <u>study number</u> at the top of your work.

Also write whether you were a <u>member</u> of the elastic team, plastic team or no team.



Figure 1. Frame structure

Figure 2. Yield contours

Problem 1

A frame consists of four members (Fig.1). The columns have a strength $2M_p$. The roof members have a strength M_p . The members are rigidly connected. The supports are fixed. The structure is loaded by an evenly distributed line load *q* per column length (wind load). The relation of Figure 2 exists between the plastic moments and the plastic normal forces.

$$N_{p} = \beta \frac{M_{p}}{a}$$

The influence of shear on the yield contour is neglected. Buckling and second order effects are not considered.

- **a** Assume $\beta \to \infty$. Determine the collapse load *q* for all possible mechanisms. Write the collapse loads as functions of M_p and *a*. What is the decisive collapse load? (1.5 point)
- **b** Assume $\beta \rightarrow \infty$. Draw the bending moment diagram and normal force diagram for the structure at the moment of collapse. (1.5 points)
- **c** Assume β = 6. Choose one of the following problems (You need not do both). – Determine the largest <u>lower-bound</u> for *q*.

- Determine the smallest <u>upper-bound</u> for q.

You only need to write down the equations and not solve the equations (1.5 points).

Problem 2

A reinforced concrete plate has simply supported edges and free edges (Fig. 3). It carries an evenly distributed load p [kN/m²]. There is no other load on the plate. The plate is homogeneous and orthotropic.



Figure 3. Plate dimensions and reinforcement

a Consider the yield line patterns of Figure 4. Which of these patterns give kinematically possible mechanisms? (1 point)



Figure 4. Yield line patterns of problem 2a

b Consider the yield line pattern of Figure 5. Determine an <u>upper-bound</u> for *p* expressed in m_p and *a* (1.5 point).



Figure 5. Mechanism of Problem 2b

c Determine the largest <u>lower-bound</u> for *p* using torsion free beams ($m_{xy} = 0$). You only need to write down the equations and not solve the equations. (1.5 point)

Problem 3

- **a** What is ductility? Choose A, B, C or D. (0.5 point)
 - A The ability to deform with much plastic deformation.
 - B Plastic deformation in the direction of the normal vector on the yield contour.
 - C The material property that the complementary energy remains positive semi-definite.
 - D Shifting of the yield contour during strengthening of yielding.
- **b** Welding stresses and rolling stresses reduce the ... Choose A, B, C or D. (0.5 point)
 - A ... moment capacity of a beam cross-section.
 - B ... buckling load of a column.
 - C ... deflection of a beam.
 - D ... interaction of plastic moment en normal force in a column cross-section.
- **c** The yield lines in a plate transfer bending moments and torsion moments. The torsion moments are neglected in calculating the virtual work. Why? Choose A, B, C or D. (0.5 point)
 - A The stresses due to the torsion moments are very small.
 - B The torsion moments must be zero because the material has no extra capacity.
 - C A rhombic interaction diagram is assumed.
 - D There is no torsion deformation in a yield line.



Answer to problem 1b





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> b:=6: Np:=b*Mp/a:
> eq1:= V1*2/sqrt(5.) + (1-a1)*2*Np*1/sqrt(5) + V2*1/sqrt(5) - (1-a2)*Np*2/sqrt(5) =0:
> eq2:= V1*1/sqrt(5) - (1-a1)*2*Np*2/sqrt(5) + V2*2/sqrt(5) + (1-a2)*Np*1/sqrt(5) =0:
> eq3:= a1*2*Mp = (1-a2)*Np*2/sqrt(5)*6*a - (1-a2)*Np*1/sqrt(5)*6*a - V2*1/sqrt(5)*6*a - V2*
 2/sqrt(5)*6*a - a2*Mp:
>
> eq4:= (1-a2)*Np*2/sqrt(5) - V2*1/sqrt(5) - (1-a3)*Np*2/sqrt(5) - V3*1/sqrt(5) =0:
> eq5:=-(1-a2)*Np*1/sqrt(5) - V2*2/sqrt(5) - (1-a3)*Np*1/sqrt(5) + V3*2/sqrt(5) =0:
> eq6:= a2*Mp = V3*2*sqrt(5)*a - a3*Mp:
>
> eq7:= (1-a3)*Np*2/sqrt(5) + V3*1/sqrt(5) - (1-a4)*2*Np*1/sqrt(5) + V4*2/sqrt(5) - q*6*a =0:
> eq8:= (1-a3)*Np*1/sqrt(5) - V3*2/sqrt(5) - (1-a4)*2*Np*2/sqrt(5) - V4*1/sqrt(5) + q*3*a =0:
> eq9:= a3*Mp = V4*2*sqrt(5)*a - q*3*sqrt(5)*a*1/2*sqrt(5)*a - a4*2*Mp:
>
> solve({eq1,eq2,eq3,eq4,eq5,eq6,eq7,eq8,eq9},{q,a1,a2,a3,a4,V1,V2,V3,V4});
 a
                           a
                                            a
  = 0.8971709740, a3 = 0.9188574688, a4 = 0.9878468028, q = \frac{0.2512774836 Mp}{2}
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Answer to problem 2a

B, D, F

3 or less correct	0.0 point
4 correct	0.3 point
5 correct	0.7 point
6 correct	1.0 point

Answer to problem 2b

> E:= p*5*a*5*a/2*w/3 + p*5*a*6*a*w/2 + p*2*a*4*a*w/2;

$$E := \frac{139}{6} p a^2 w$$

> A:= mp*5*a*w/(5*a) + mp*5*a*w/(5*a) + mp*5*a*w/(5*a) + 3*mp*4*a*(w/(2*a)+w/(5*a)); $A := \frac{57}{5} mp w$

> p:=solve(E=A,p); evalf(p);

$$p := \frac{342}{695} \frac{mp}{a^2}$$
0.4920863309 mp





Answer to problem 3

- a A
- b B
- c B (D is not correct for yield lines that are inclined to the reinforcement directions: the bending rotation in one direction is the torsion rotation in the other.)

